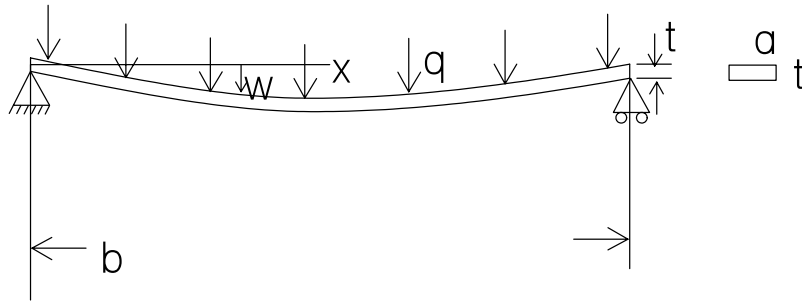


Plate Bending Introduction

see: bending with z load sheet for derivations

review general beam, simply supported, clamped long plate
long plate, boundary conditions (end restrained) not so long plate

simply supported beam:



$$Q(x) := \frac{q \cdot b}{2} - q \cdot x$$

$$\int_0^x Q(\xi) d\xi \rightarrow \frac{1}{2} \cdot q \cdot b \cdot x - \frac{1}{2} \cdot q \cdot x^2$$

$$M(x) := \frac{q \cdot b}{2} \cdot x - q \cdot x \cdot \frac{x}{2} \quad \frac{d}{dx} M(x) \rightarrow \frac{1}{2} \cdot q \cdot b - q \cdot x = 0 @ x = b/2$$

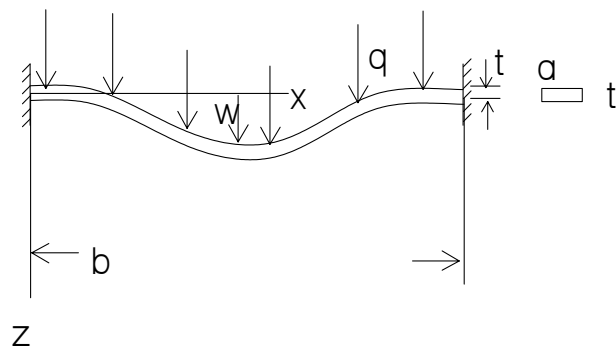
$$M\left(\frac{b}{2}\right) \rightarrow \frac{1}{8} \cdot q \cdot b^2 \quad M_{\max} := \frac{1}{8} \cdot q \cdot b^2$$

$$\sigma_x := \frac{M(x)}{I} \cdot z \quad \text{maximum when } z = t/2, m(x) = M_{\max} \quad \sigma_{x_max} := \frac{M_{\max}}{I} \cdot \frac{t}{2}$$

$$I := \frac{1}{12} \cdot t^3 \cdot a \quad \sigma_{x_max} := \frac{\frac{1}{8} \cdot q \cdot b^2}{I} \cdot \frac{t}{2} \quad \sigma_{x_max} \rightarrow \frac{3}{4} \cdot q \cdot \frac{b^2}{t^2 \cdot a}$$

+ tension other side of load
- compression on load side

clamped beam:



need to use deflection $\frac{d^4}{dx^4} w$ to solve

result:
$$M(x) := -q \cdot \left(\frac{x^2}{2} - \frac{b \cdot x}{2} + \frac{b^2}{12} \right)$$

Given $\frac{d}{dx} M(x) = 0$ Find(x) $\rightarrow \frac{1}{2} \cdot b$

$$M\left(\frac{b}{2}\right) \rightarrow \frac{1}{24} \cdot q \cdot b^2$$

$$M_{\max} = M(x = 0, x = b)$$

$$M(0) \rightarrow \frac{-1}{12} \cdot q \cdot b^2 \quad M(b) \rightarrow \frac{-1}{12} \cdot q \cdot b^2$$

$$\sigma_{x_max} := \frac{\frac{1}{12} \cdot q \cdot b^2}{I} \cdot \frac{t}{2}$$

$$\sigma_{x_max} \rightarrow \frac{1}{2} \cdot q \cdot \frac{b^2}{t^2 \cdot a}$$

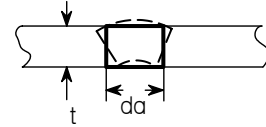
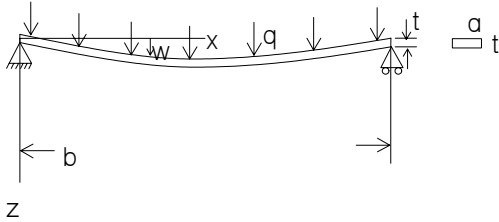
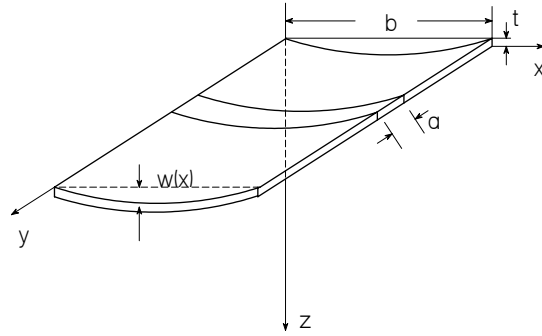
- compression other side of load

+ tension on load side

long plate:
treating unit length (away
from end effects)

$$a := 1$$

section at a
simply supported
free to pull in



strain in y constrained by adjacent plate
anticlastic curvature $R_v = 1/\nu * R$

$$\epsilon_x := \frac{\sigma_x}{E} - \frac{\nu \cdot \sigma_y}{E} \quad \epsilon_y := \frac{\sigma_y}{E} - \frac{\nu \cdot \sigma_x}{E} = 0 \quad \frac{\sigma_y}{E} = \frac{\nu \cdot \sigma_x}{E}$$

substituting $\Rightarrow \epsilon_x := \frac{(1 - \nu^2) \cdot \sigma_x}{E}$ or ... $\epsilon_x := \frac{\sigma_x}{\frac{E}{(1 - \nu^2)}}$ or ... $\epsilon_x := \frac{\sigma_x}{E'}$ where $E' = E/(1 - \nu^2)$

rearranging $\Rightarrow \sigma_x := \frac{E \cdot \epsilon_x}{1 - \nu^2}$ as in bending of beam: $\epsilon_x := -\frac{z}{R} \Rightarrow \sigma_x := -\frac{E}{1 - \nu^2} \cdot \frac{z}{R}$

$$M := \int_{-t/2}^{t/2} \sigma_x \cdot z \, dz \quad M := - \int_{-t/2}^{t/2} \frac{E \cdot z}{1 - \nu^2} \cdot \frac{d^2}{dx^2} w \cdot z \, dz \quad M := -\frac{E \cdot t^3}{12 \cdot (1 - \nu^2)} \cdot \frac{d^2}{dx^2} w \quad M = M_y$$

define: $D := \frac{E \cdot t^3}{12 \cdot (1 - \nu^2)}$ $M := -D \cdot \frac{d^2}{dx^2} w$

moment relationships are the same:
simply supported:

clamped: (figure not shown)

$$\sigma_{x_max} := \frac{1}{8} \cdot q \cdot b^2 \cdot \frac{t}{I}$$

$$\sigma_{x_max} := \frac{3}{4} \cdot q \cdot \frac{b^2}{t}$$

$$\sigma_{x_max} := \frac{1}{12} \cdot q \cdot b^2 \cdot \frac{t}{I}$$

$$\sigma_{x_max} := \frac{1}{2} \cdot q \cdot \frac{b^2}{t}$$

Hughes 9.1.7 is of the form:

$$\sigma_{x_max} := k \cdot q \cdot \frac{b^2}{t}$$

$k = 0.75$ simply supported

$k = 0.5$ clamped

N.B. stress is + & - from bending

