

11/00

## IV-7 BEARING CAPACITY

### 1. Bearing Capacity of Shallow Foundations

Page No

1.1 Effect of Cohesion: Strip Footing

1

1.2 Shape Factor for Cohesion

1

1.3 Physical Significance of  $c$ ,  $\phi$  &  $\gamma$ 

1

1.4 Effect of Water Table: Cohesionless; No Capillarity

2

1.5 Effect of Water Table: Cohesive; Full Capillarity

2

13-782 500 SHEETS 11 L x 17 L 5 SQUARE  
 42-381 50 SHEETS 11 L x 17 L 5 SQUARE  
 42-382 100 SHEETS 11 L x 17 L 5 SQUARE  
 42-383 200 SHEETS 11 L x 17 L 5 SQUARE  
 42-384 100 RECYCLED WHITE 5 SQUARE  
 42-385 200 RECYCLED WHITE 5 SQUARE  
 Made in U.S.A.

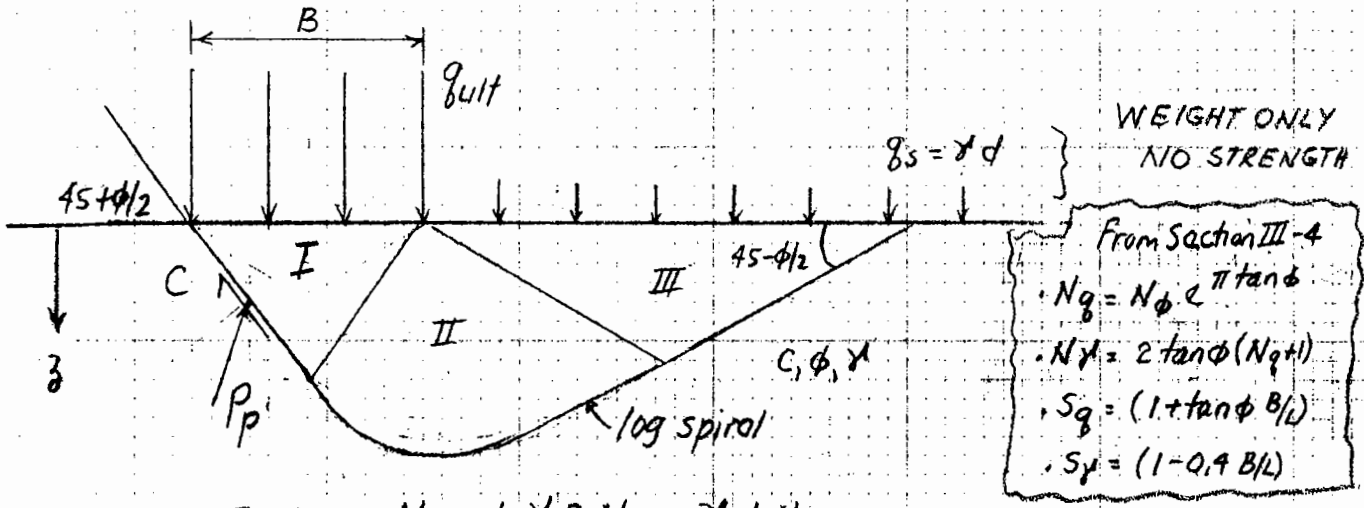


Part IV-7 BEARING CAPACITY

1. BEARING CAPACITY OF SHALLOW FOUNDATIONS

1.1 Effect of Cohesion: Strip Footing

Modified Terzaghi = Vesic (1973: JSMFD, ASCE, V99, 5M1)



$q_{ult} = c N_c + \frac{1}{2} \gamma B N_\gamma + \gamma d N_q$

$N_c = \cot \phi (N_q - 1)$  For  $\phi = 0$ ,  $N_c = 2 + \pi = 5.14$ ,  $N_q = 1$ ,  $N_\gamma = 0$

1.2 Shape Factor for Cohesion (Vesic, 1973)

$S_c = [1 + \frac{B}{L} \frac{N_q}{N_c}]$  For  $\phi = 0$ ,  $S_c = 1.2 \rightarrow N_c = 6.2$  Approximate  
 $B/L = 1.0$

1.3 Physical Significance of  $c, \phi, \gamma$

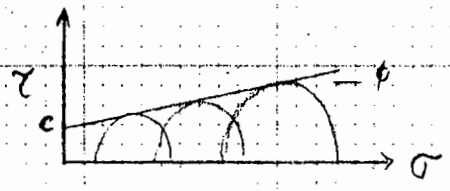
SOURCE OF SIGNIFICANT ERRORS IN PRACTICE!

\* These should relate the strength of the soil to the magnitude of the initial stress gradient and the applied stresses from footing

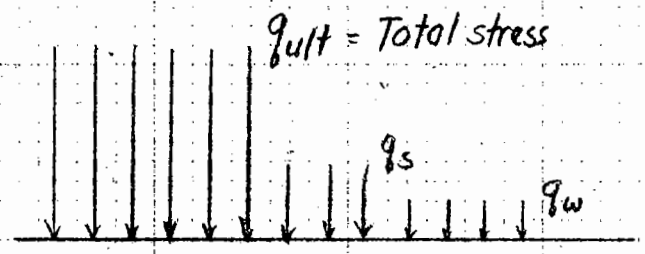
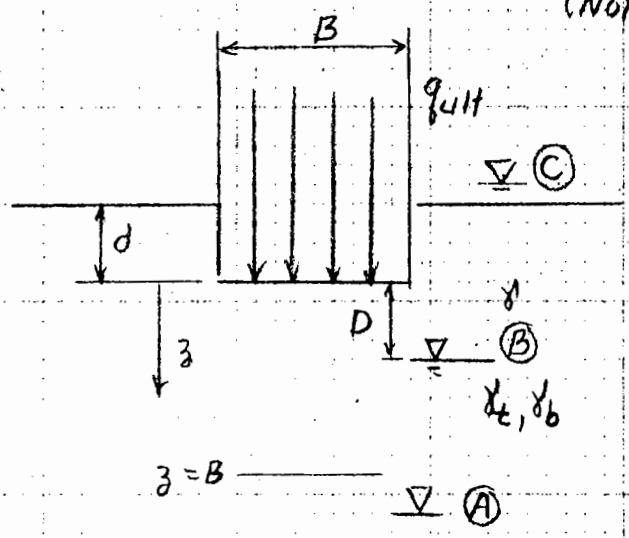
- 1) Drained loading ( $\Delta u = 0$ ) - Use  $c =$  \_\_\_\_\_  $\phi =$  \_\_\_\_\_  
 $\gamma =$  \_\_\_\_\_  $\rightarrow z$  vs  $\Delta \sigma'_v$  and  $q_s = \sigma'_v$  at  $z = 0$  for  $x \geq 0$ .
- (2) Undrained loading of saturated clay ( $\Delta w = 0, B = 1.00$ )  
 $c =$  \_\_\_\_\_  $\phi =$  \_\_\_\_\_  $N_\gamma =$  \_\_\_\_\_  $q_{ult} \text{ (strip)} =$  \_\_\_\_\_  
 $N_q =$  \_\_\_\_\_ " ( $B \neq L$ ) = \_\_\_\_\_

(3) Undrained loading,  $S < 100\%$

UU tests  $\rightarrow c \neq \phi, \gamma = \gamma_t$   
 (Covered later)



1.4 Effect of Water Table: Cohesionless: No Capillarity  
 (Not in LIW text)



- $\bar{\gamma} = \bar{\gamma} \rightarrow \Delta \sigma'_v \text{ vs } z$
- $q_s = \sigma'_v$  due wgt soil above footing
- $q_w = u$  at  $z = 0$

Strip  $q_{ult} = \frac{1}{2} \bar{\gamma} B N_\gamma + q_s N_q + q_w$

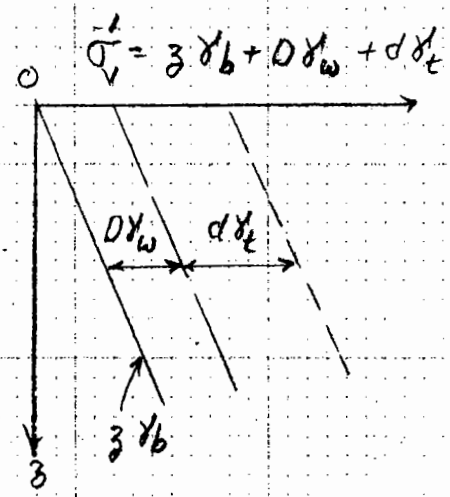
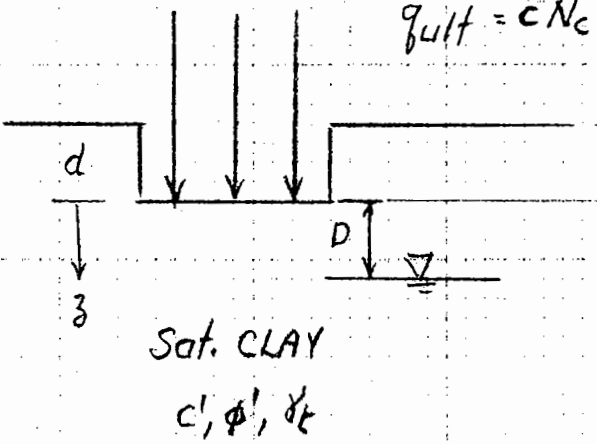
- Case (A) ( $D > B$ )  $\bar{\gamma} = \underline{\hspace{2cm}}$   $q_s = \underline{\hspace{2cm}}$   $q_w = \underline{\hspace{2cm}}$
- Case (C) (Negative D)  $\bar{\gamma} = \underline{\hspace{2cm}}$   $q_s = \underline{\hspace{2cm}}$   $q_w = \underline{\hspace{2cm}}$
- Case (B) ( $D < B$ )  $\bar{\gamma} = \underline{\hspace{2cm}}$

Note: Rising WT  $\rightarrow$  decrease in  $q_{ult}$

1.5 Effect of Water Table: Cohesive: Full Capillarity (Strip)

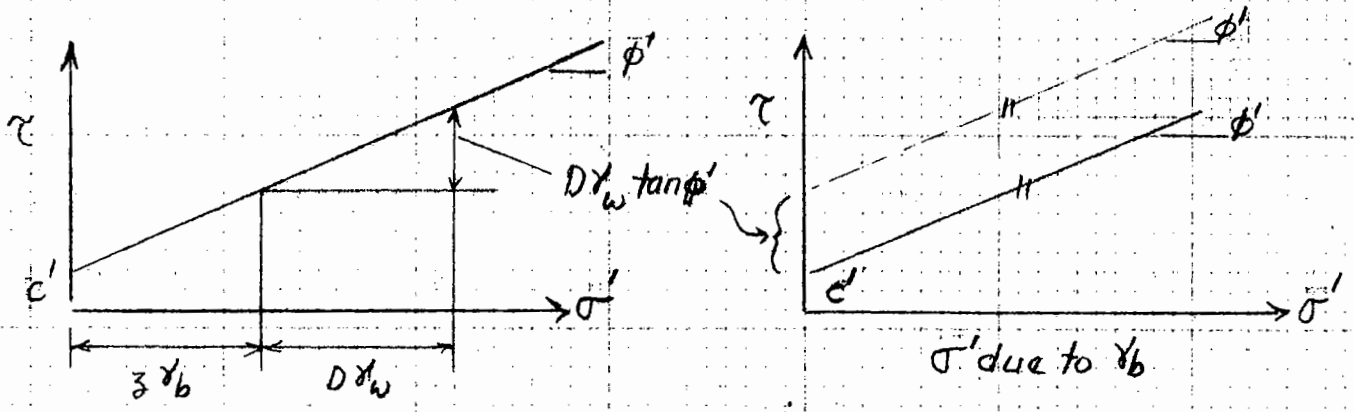
(i) Problem

$q_{ult} = c' N_c + \frac{1}{2} \gamma_b B N_\gamma + d \gamma_t N_q + [\text{Effect of } D \gamma_w = \Delta q_{ult}(c)]$



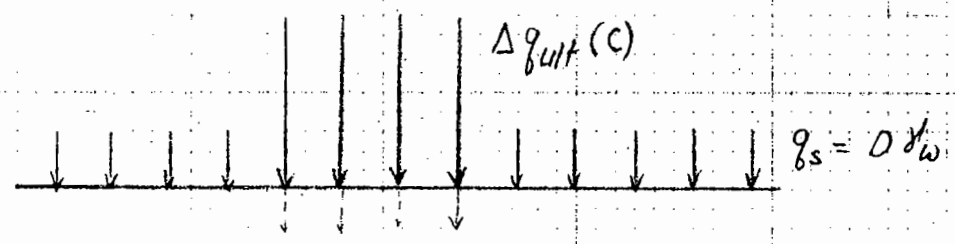
Part IV-7 BEARING CAPACITY

(2) Apparent "cohesion" = added  $\sigma'$  within clay  $\rightarrow$  increased strength



$\Delta q_{ult}(c) =$  \_\_\_\_\_

(3) Treat as "surcharge"



$\Delta q_{ult}(c) = D\gamma_w N_q$  - Why incorrect? What did CCL forget?

Actual  $\Delta q_{ult}(c) =$  \_\_\_\_\_

(4) Do (3) & (4) check?